AAPA's 14th International Flexible Pavements Conference

Sydney 25–28 September 2011

Topic: Laboratory versus Field Assessment of Full Depth Asphalt Mixes in New Zealand.

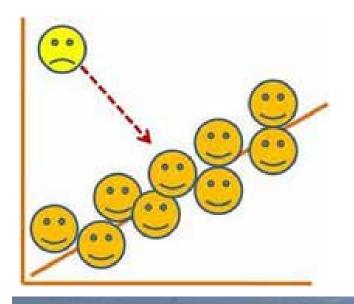
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Position: Technical Manger Auckland, TS Manager Auckland

Organisation: Fulton Hogan

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If it is not Representativedon't use it!

Outline

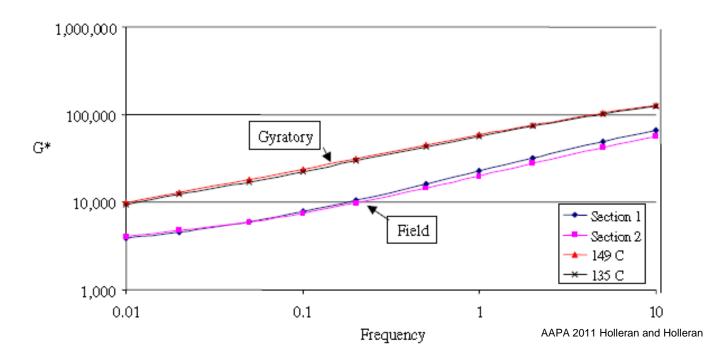
- Considerations in Performance of Asphalt Laboratory Design V Field
- RAP use and its issues
- Project Results Field v laboratory
- How do we get design to field reliably?



You cant afford to ignore basic principles

Differences due to compaction - Lit

- Void distribution
- Particle orientation
- Physical properties



Void distribution lab V Field – Image analysis

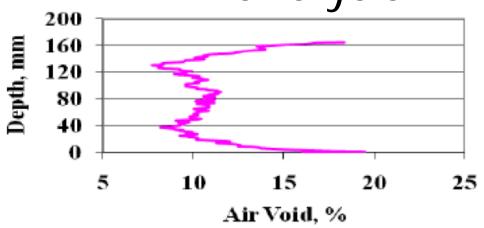
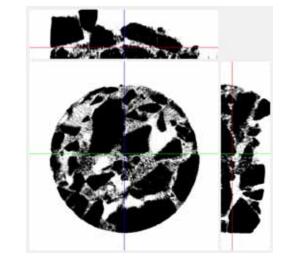
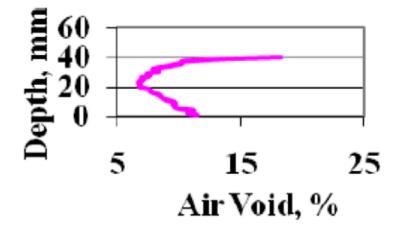


Figure Example of lab compacted sample





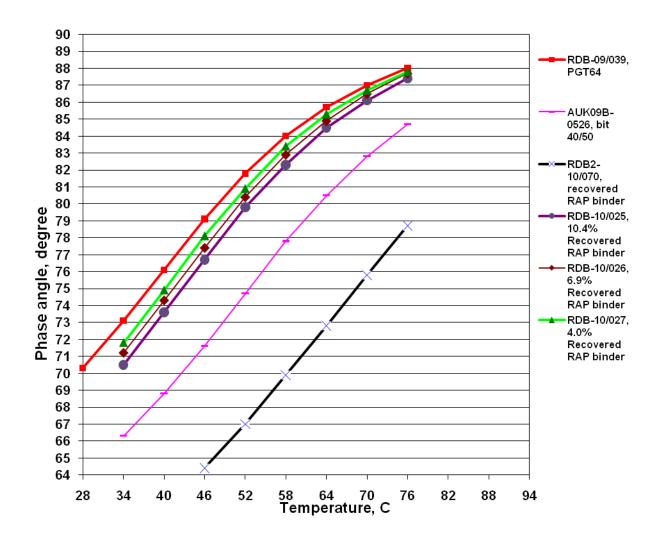
Design

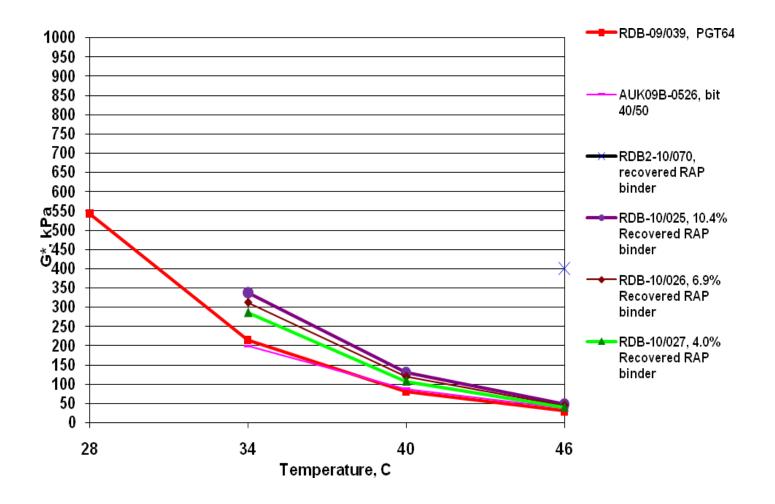
- Binder allowance and mixing
- Grading- accessible fines
- Sg testing- Volumetrics are sensitive to Rap Sg
 Calculate from fine and coarse extracted aggregates and allowance for binder
- Handling RAP not changing binder

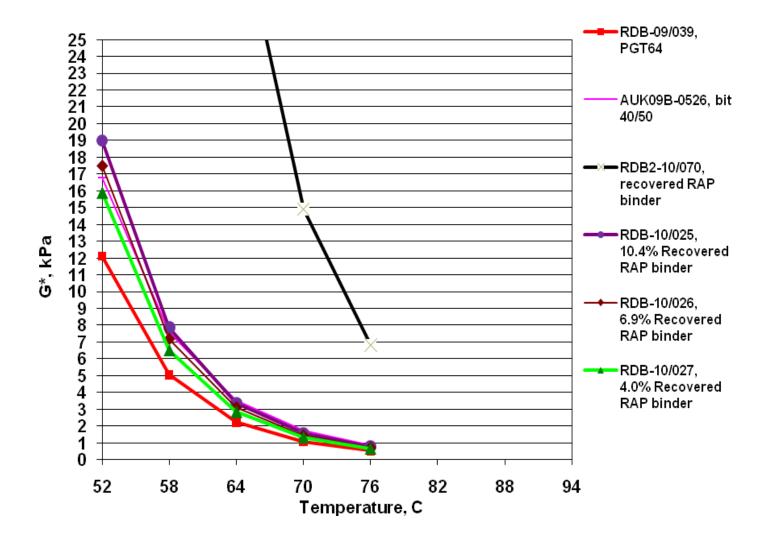
Contribution of RAP binder

- 100% mixing does not occur as physical properties are not consistent with this.
- Partial mixing occurs and may create an interfacial area between virgin and aged binder
- The interface will depend on
 - the virgin binder rheology and
 - Temperature
 - Mixing time
 - Heat soak time silo storage
 - Degree of aging in the recovered binder

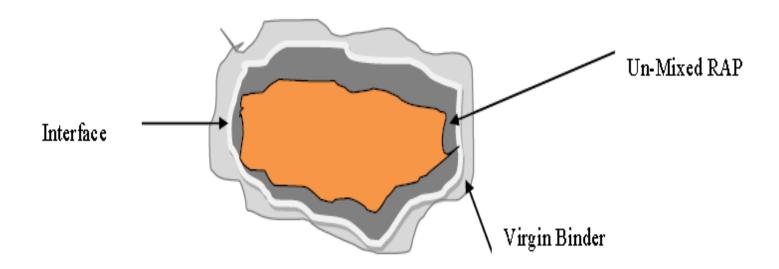








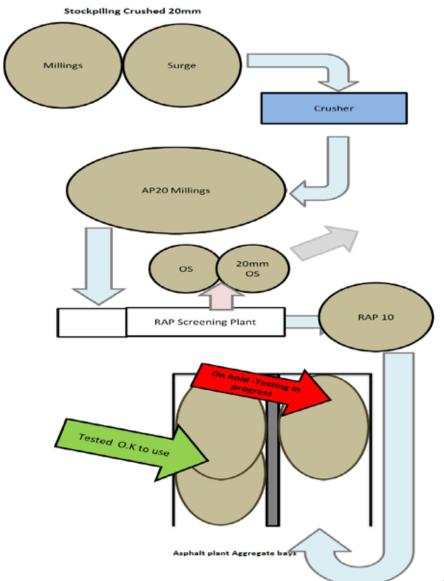
Conceptual Effect of Rap Binder



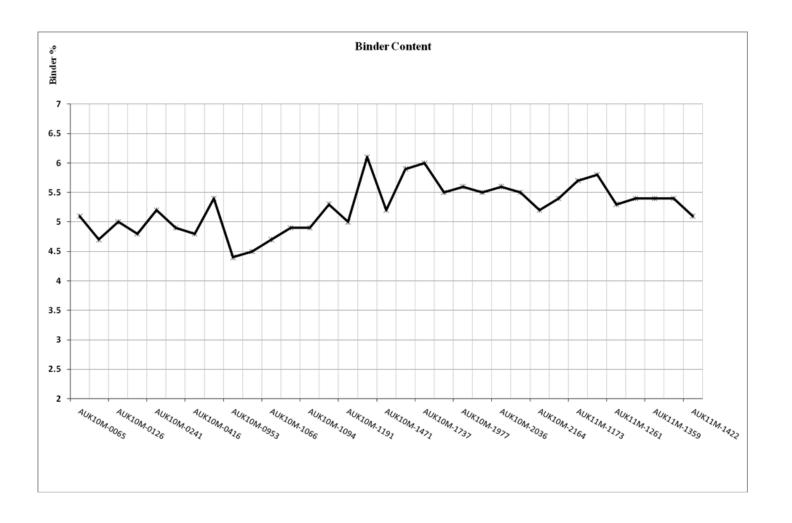
Effect of Non accessible Fines



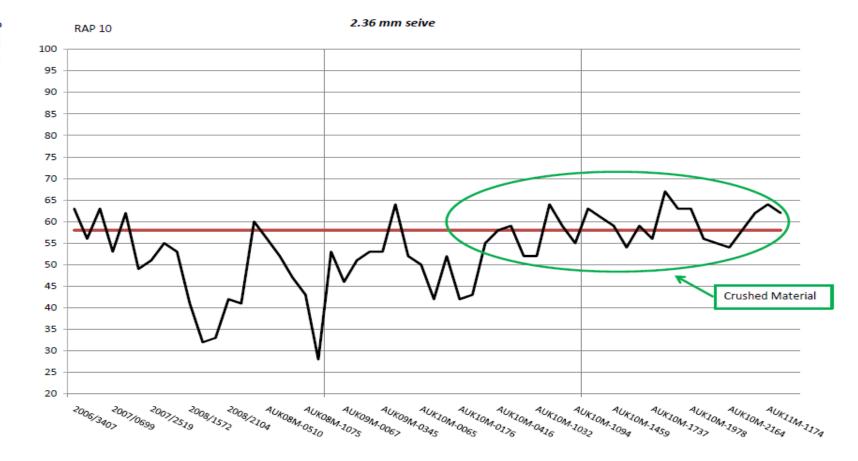












Region RAP Type Process	Silverdale RAP 7 Screened		Auckland RAP10 Screened		Auckland RAP10 Crushed	
	Binder	Cont Sieve (2.36mm)	Binder	Cont Sieve (2.36mm)	Binder	Cont Sieve (2.36mm)
Number of Tests	32.0	32.0	46.0	46.0	14.0	14.0
Average	5.3	65.9	5.4	51.9	5.5	59.1
Max	6.2	78.0	6.4	64.0	5.8	67.0
Min	4.4	39.0	4.4	28.0	5.1	53.0
Std Dev	0.5	8.0	0.5	8.6	0.2	4.0
2x Std Dev	1.0	16.0	1.1	17.2	0.4	9.2





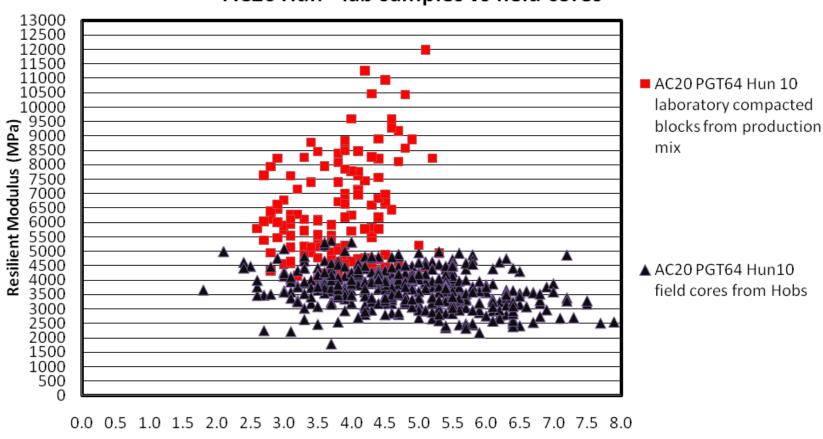




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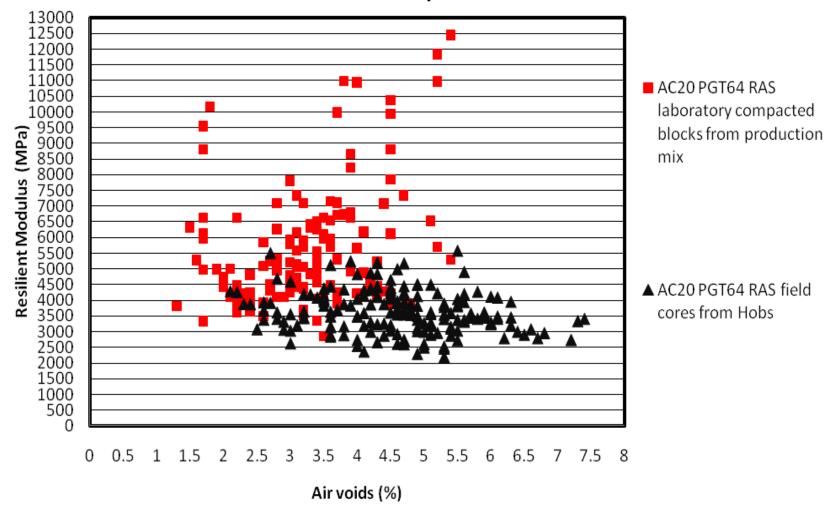
Laboratory Compaction V Field



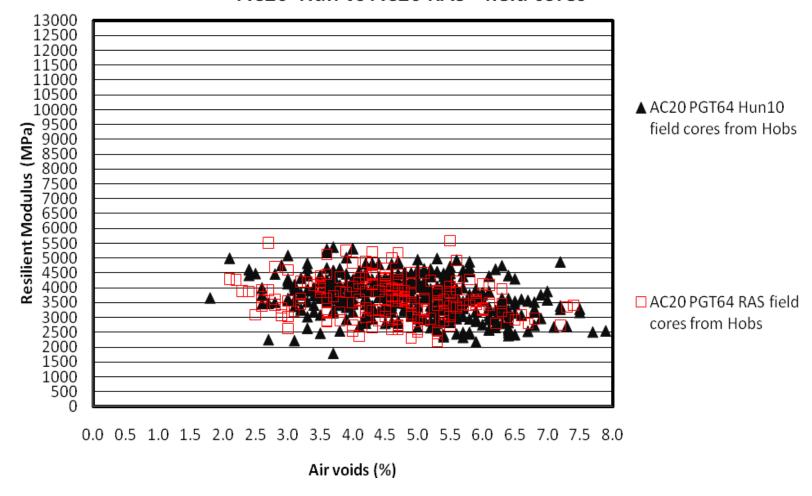


Air voids (%)

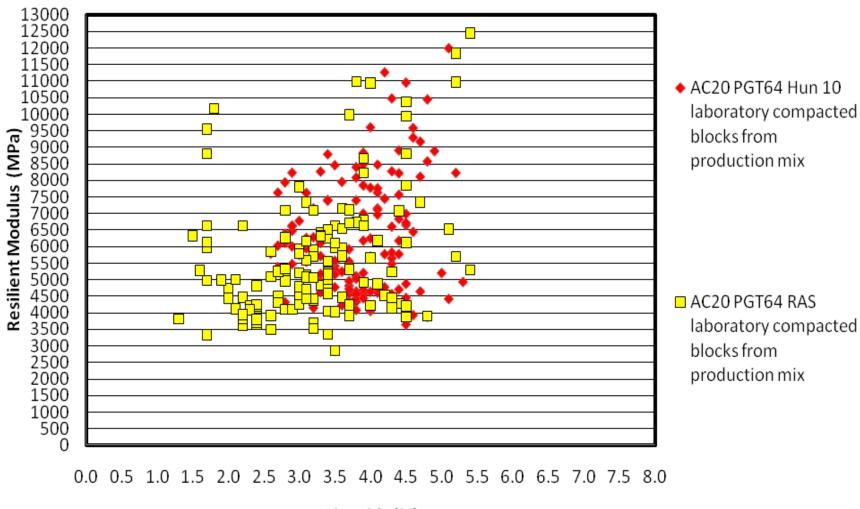
AC20 RAS - lab samples vs field cores



AC20 Hun vs AC20 RAS - field cores



AC20 Hun vs AC20 RAS - lab samples

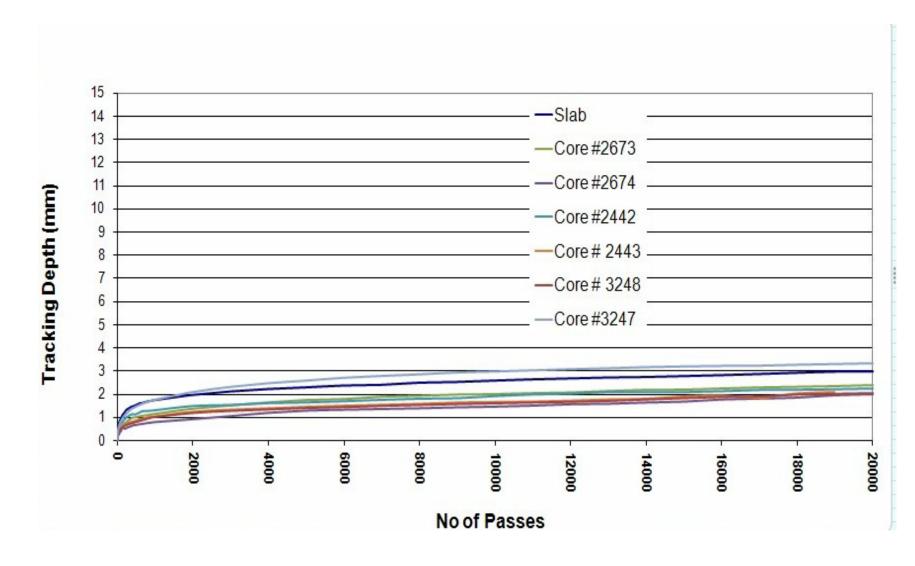


Air voids (%)

Matta results



- Field Matta consistently lower for field samples
- Less scatter
- Void ranges similar but wider for field samples
- Density levels of compaction within same ranges as laboratory
- Difference maybe due to void distribution and particle orientation
- Pavement design with Matta should not be done with laboratory design if Servo used



Wheel tracking

- Compacted with a wheel system
- Slabs very similar though not identical to field samples
- Differences may be due to different void distribution and orientation



Mix	15% RAP	15% RAP	Typical	Virgin Mix
	Mix Cycles to	Mix Flexural	Virgin Mix AC20	Flexural Strength
	Fail	Strength (MPa)	Cycles to Fail	(MPa)
AC20Hun	250,000	6560	315,000	6760
AC20RAS	319,480	4500	315,000	6760

^{*}Fatigue Life of Compacted Bituminous Mixes Subject to Repeated AG:PT/T233 - 06 Flexural Bending 20 $^{\circ}$ C 400 ms.

Fatigue Life

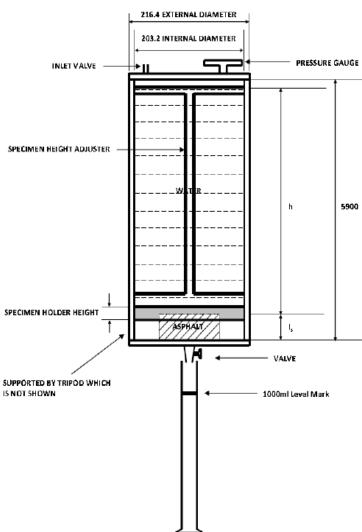
- 4 pt beam testing
- Comparable fatigue life to virgin mixes
- Mix and aggregate dependant
- Fatigue beams made using rolling wheel so may be more reflective of in field performance



LABORATORY PERMEAMETER

DIMENSIONS IN MILLIMETERS





Permeability

- Greater for field compacted mixes
- AC20 compacted to optimum is not impermeable
- Gyratory compacted samples in lab have lower permeability than field cores of similar void levels

Conclusions

- RAP can be successfully controlled but requires special handling in design and in processing
- Lab Matta results do not reflect field results and should not be used for design
- Gyratory compaction does not simulate field compaction
- Wheel compaction should be considered as it gives better reproduction of field core results
- QC is better done on field cores than on production samples in the lab – Filed takes all variables into account