

# AAPA's 14<sup>th</sup> International Flexible Pavements Conference

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## **Design of Recycled Asphalt Mixtures**

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- **Recycling of hot mix asphalt in the Netherlands**
  - **Mix design issues of mixtures with high RAP content; effects of moisture and mixing method**
  - **Effect of RAP content, moisture and mixing method on mechanical characteristics**
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# Some Statistics about the Netherlands



**16 million people**  
**3 million tons of RAP**  
**15 million tons of CDW**  
**No natural aggregates**  
**No space for dumping waste**  
**Recycling is a must**

# Government policy

- **Recycling is a must.**
- **Costs per ton for dumping RAP are very high, close to costs of producing new mixture.**
- **Active policy in development of techniques, specifications, test methods etc.**
- **Since 1990, recycled asphalt mixtures are in the Dutch standards.**
- **Since 1990, RAP is treated as “normal” material.**

# Some early Developments

- **1976 Renofalt process; recycling with up to 100% RAP**
- **1990 MARS process; recycling with up to 100% RAP**

# State of the Art Recycling in the Netherlands

- Asphalt production of  $9 \cdot 10^6$  ton/year mostly for binder and surface layers.
  - Consumption of bitumen  $0.37 \cdot 10^6$  ton/year.
  - At the moment  $3.5 \cdot 10^6$  ton/year of RAP.
  - 80 % of the RAP is used in hot mix.
  - 65 % of new HMA production contains RAP.
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# State of the Art Recycling in the Netherlands

- Recycling in STAC (base layer) maximum 50 %.
- Recycling in OAC (binder layer) maximum 50%.
- Recycling in DAC (wearing course) maximum 50%.
- Recycling in Porous Asphalt (wearing course) maximum 20 %.
- No recycling in SMA.
- Log pen rule is used for the combined penetration (old –new bitumen) in the mix design for all mixes.

$$a \log \text{pen}_{\text{RAP}} + b \log \text{pen}_{\text{virgin}} = (a + b) \log \text{pen}_{\text{mix}}$$
$$a + b = 1$$



# Hot Mix Asphalt plants (partial recycling PR) in the Netherlands

Type of plant	Number
Batch plant with parallel drum	38
Batch plant with cold RAP feed	1
Drum mixer suitable for PR	5
Double barrel drum	1
<b>Total</b>	<b>45</b>

# Some Issues

- From max 50% to 70% recycling in base layers
  - Many PA layers are to be replaced in a first or second maintenance cycle. This is RAP with extremely hard bitumen (pen < 15).
  - How to keep the temperature of virgin aggregate at reasonable level at higher RAP contents.
  - For surface layers, requirements PSV stone are increased (>57). Is aggregate in current RAP good enough?
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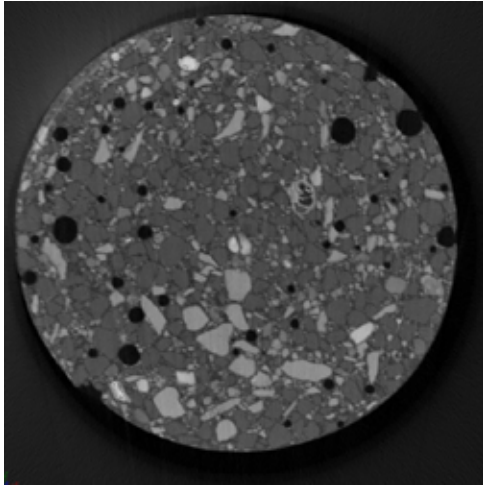
# CE marking effective since January 2009

- **Functional requirements in CE marking also for RAP mixtures:**
    - **water sensitivity (retained ITS),**
    - **stiffness (4 point bending),**
    - **fatigue (4 point bending),**
    - **permanent deformation (triaxial test).**
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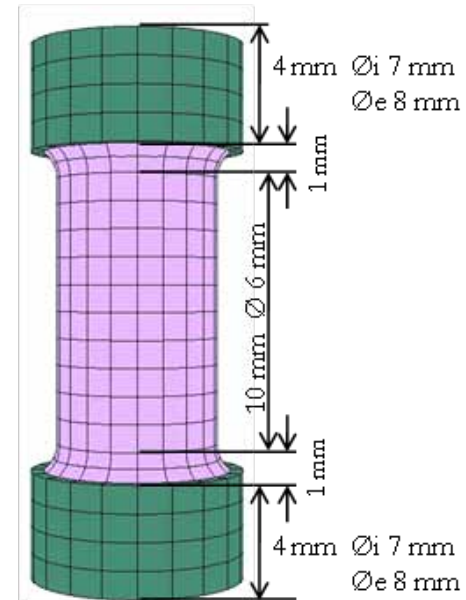
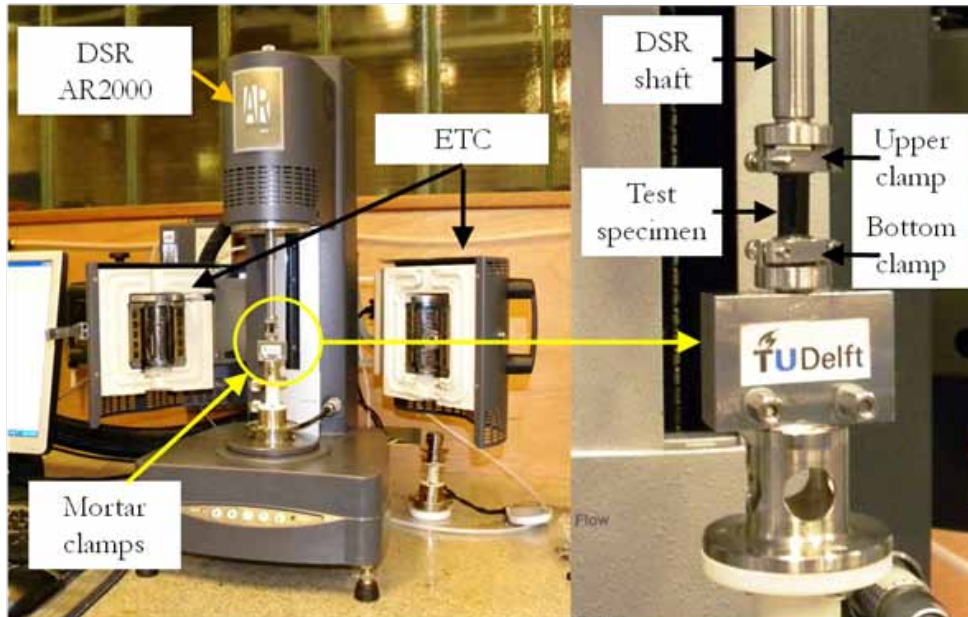
# Important Research questions

- **Fatigue properties of mixtures with very high RAP contents.**
  - **Healing of mixes with RAP.**
  - **How to recycle mixtures with PMB (can log (pen) rule be used).**
  - **Re-use of Porous Asphalt RAP .**
  - **More general: increase amount of RAP in the top layers**
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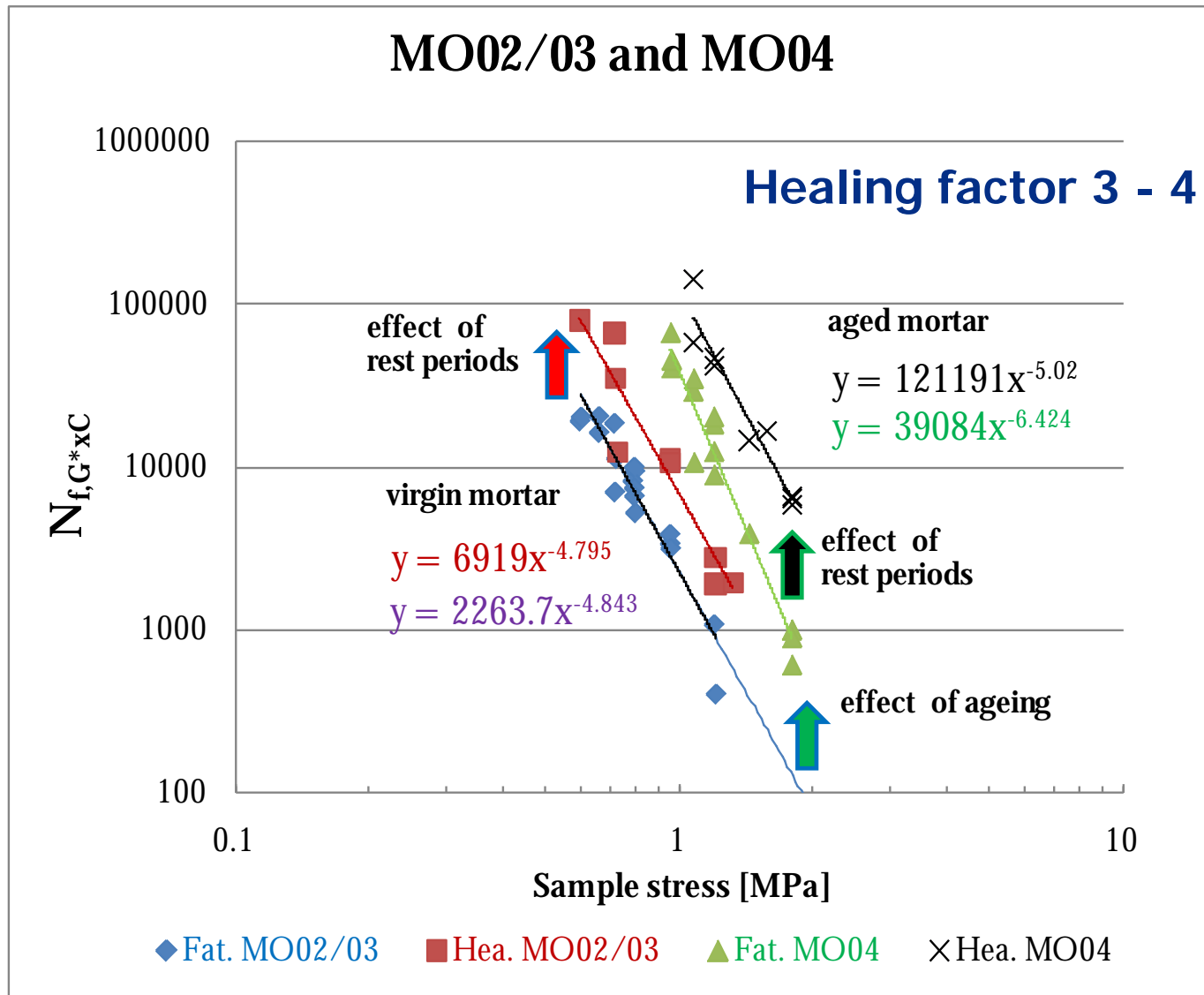
# Fatigue and Healing Tests



Mortar samples  $h = 10 \text{ mm}$   $f = 6 \text{ mm}$   
Mortar means all aggregates  
with  $f < 0.5 \text{ mm}$  + bitumen



# Fatigue and Healing Results of Virgin and Aged Mortars



# Consequences for Recycling

- Aged mortar has better fatigue resistance than virgin mortar
- Ageing does seem to have bad effect on healing

**CAREFUL!!!!**

- Results are obtained on artificial aged mortars
  - However, rheological and chemical characteristics of artificial aged binder were the same as those of binder extracted from RAP
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# Comparison Lab vs “Field”

- Rheological properties of lab aged binder = rheological properties of binder recovered from RAP
  - Chemical composition lab aged binder = chemical composition binder from RAP
  - Fatigue characteristics were the same (no rest periods)
  - Healing lab aged mortar 3 – 4
  - Healing mortar with RAP (field aged) binder 1.8
  - In all cases mortar aggregates were the same
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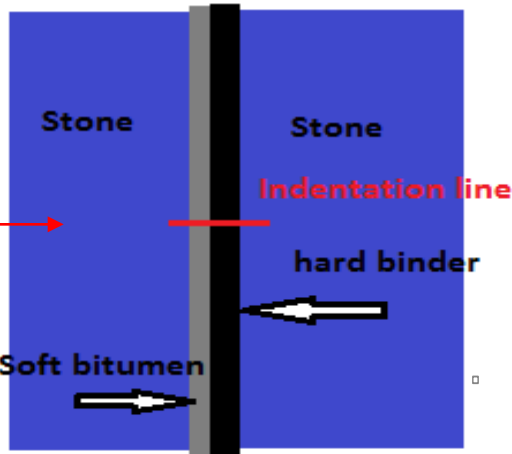
# Blending



**Will this binder blend  
with new binder?**

**"naked" stone  
"black rock" ?**

# Nano-indentation Tests to measure Blending

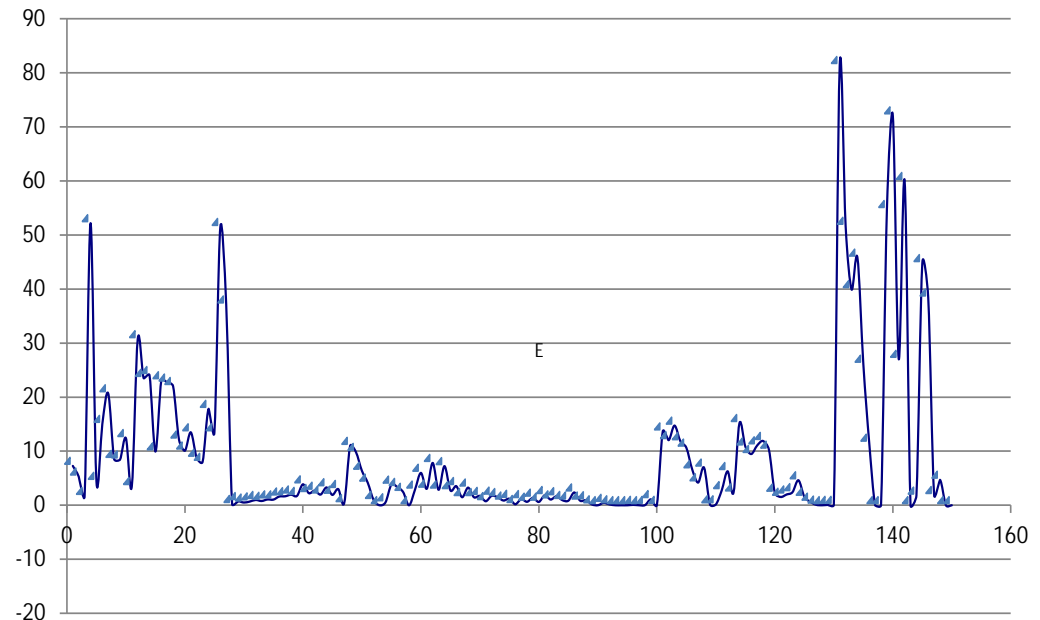


Hard binder 20/30  
Soft binder 160/200

Difficult to measure blending  
via nano-indentation

CT scanning may be the way  
to go

Modulus at Max Load



# Expectations

- **Full blending will not occur**
  - **Some kind of layered structure will develop**
  - **Fines will influence layer development**
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# Recycling in Europe

Country	Available Reclaimed asphalt mix [tons]	% re-used in hot mix	% re-used in cold mix	% of new hot mix production
Germany	14 * 10 <sup>6</sup>	82	18	60
Spain	2.25 * 10 <sup>6</sup>	8	4	3.5
Italy	14 * 10 <sup>6</sup>	18	2	
France	6.5 * 10 <sup>6</sup>	13	< 2	< 10
Norway	0.59 * 10 <sup>6</sup>	7	26	8
Netherlands	3 * 10 <sup>6</sup>	80		63

# The Problem

- Mixture design process in laboratory  $\neq$  Field conditions
  - Simulate in the lab as good as possible real mixing conditions
  - BUT CURRENTLY in the lab, RAP is preheated to same temperature as virgin materials!
  - *Field: Hot Recycling:*  
**Warm feed:** Parallel drum preheats RAP to 130 °C  
**Cold feed:** Cool and moist RAP is added to the mixing unit
  - In both cases virgin aggregates have to be heated to high temperatures
  - High temperature virgin aggregates might harm mixture quality
-

# Goals

- **Determine effects of:**
    - **amount of RAP**
    - **moisture content RAP**
    - **preheating of virgin aggregates**  
**on**
    - **mechanical characteristics of of recycled asphalt mixture.**
  - **Derive a laboratory mixture design method that simulates as close as possible the mixing procedures that are used in practice.**
-

# Virgin Materials

- Base course mixture
- Norwegian granite  $f_{\max} = 20 \text{ mm}$

Properties bitumen	Unit	Q8 pen 40/60		Q8 pen 70/100	
		Nominal values	Measured values	Nominal values	Measured values
Penetration @ 25 ° C	0.1mm	40-60	50	70-100	90
Softening point $T_{r\&b}$	° C	48-56	51	43-51	46
Penetration Index		-1	-0.96	-1	-0.45
Density at 25 ° C	kg/m <sup>3</sup>	1035	1035	1029	1029

# RAP

- 2.9 % moisture
- Crushed to maximum size of 20 mm and fractionized

**RAP was fractionized to get better control on composition**

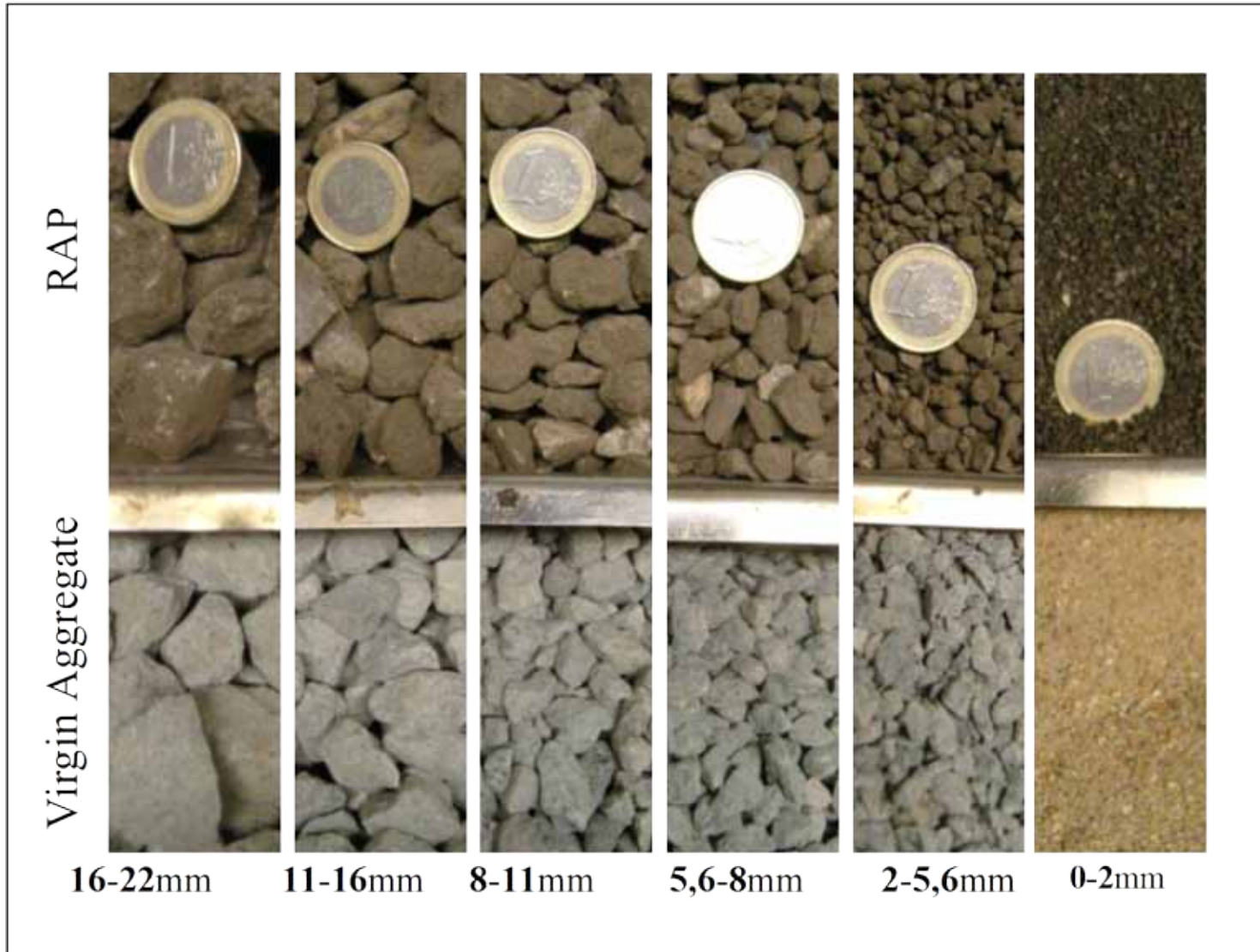
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# RAP fractions & binder content

<b>Fraction size [mm]</b>	<b>0 - 2</b>	<b>2 - 5</b>	<b>5 - 8</b>	<b>8 - 11</b>	<b>11 - 16</b>	<b>16 - 22</b>
<b>Mass percentage of total aggregate fraction</b>	<b>22</b>	<b>21</b>	<b>15</b>	<b>18</b>	<b>16</b>	<b>8</b>
<b>Percentage of binder in that fraction</b>	<b>33</b>	<b>25</b>	<b>11</b>	<b>13</b>	<b>13</b>	<b>5</b>

# RAP and Virgin Aggregates



# Mixture Compositions

Size (mm)	RAP	0% RAP	30% RAP		60% RAP		Target
		Virgin material	30 % RAP	Virgin material	60 % RAP	Virgin material	
> C22.4	0,0	1,2	0,0	1,2	0,0	1,2	1,2
C22.4 - C16	6,0	12,2	1,8	10,4	3,6	8,6	12,2
C16 - C11.2	11,0	6,6	3,3	3,3	6,6	0,0	6,6
C11.2 - C8	14,0	20,2	4,2	16,0	8,4	11,8	20,2
C8 - C5.6	9,2	7,0	2,8	4,2	5,5	1,5	7,0
C5.6 - C2	16,3	9,8	4,9	4,9	9,8	0,0	9,8
River Sand (0/2)	35,7	37,0	10,7	26,3	21,4	15,6	37,0
< 0.063	7,8	6,0	2,3	3,7	4,7	1,3	6,0
Total (%)	100,0	100,0	30,0	70,0	60,0	40,0	100,0
bitumen	4,3	4,5	1,3	3,2	2,6	1,9	4,5

# Mixing Methods

Laboratory mixing method	code	Related actual plant	Preheating conditions and temperatures (°C)		RAP	
			Virgin Agg	RAP	Moisture	Content
Standard method	SM	-	170	170	0%, 4%	0, 30, 60
Partial Warming	PW	Conventional partial warming	> 170	130	0%, 4%	30, 60
Upgraded method	UPG	Astec double barrel	>> 170	23	0%, 4%	30, 60

# Mixing Temperatures

**Final mixing temperature 170 °C**

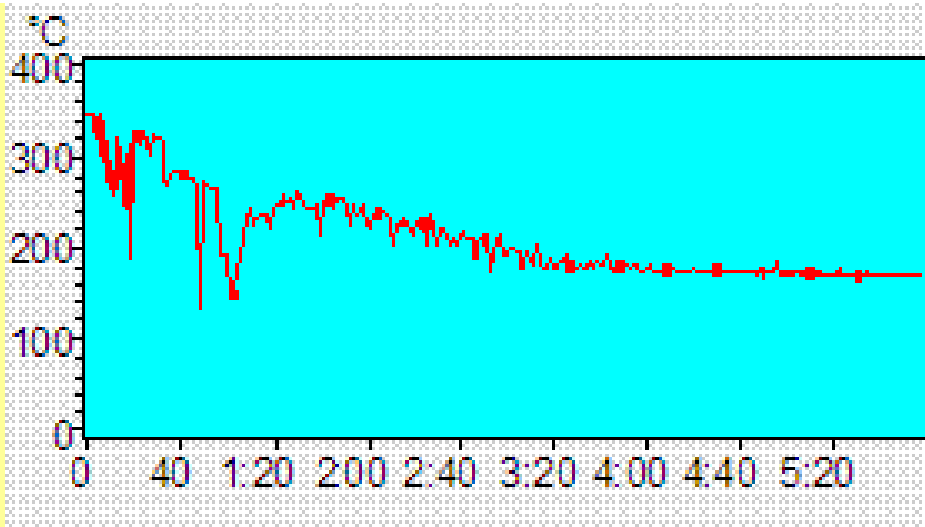
<b>Mixing method</b>	<b>Virgin aggregate preheating temp (+ 30% RAP)</b>	<b>Virgin aggregate Preheating temp (+ 60% RAP)</b>	<b>RAP preheating temp</b>
<b>SM</b>	<b>170 °C</b>	<b>170 °C</b>	<b>170 °C</b>
<b>PW</b>	<b>240 °C</b>	<b>330 °C</b>	<b>130 °C</b>
<b>UPG 0% moisture</b>	<b>290 °C</b>	<b>430 °C</b>	<b>25 °C</b>
<b>UPG 4% moisture</b>	<b>345 °C</b>	<b>515 °C</b>	<b>25 °C</b>

# Observation

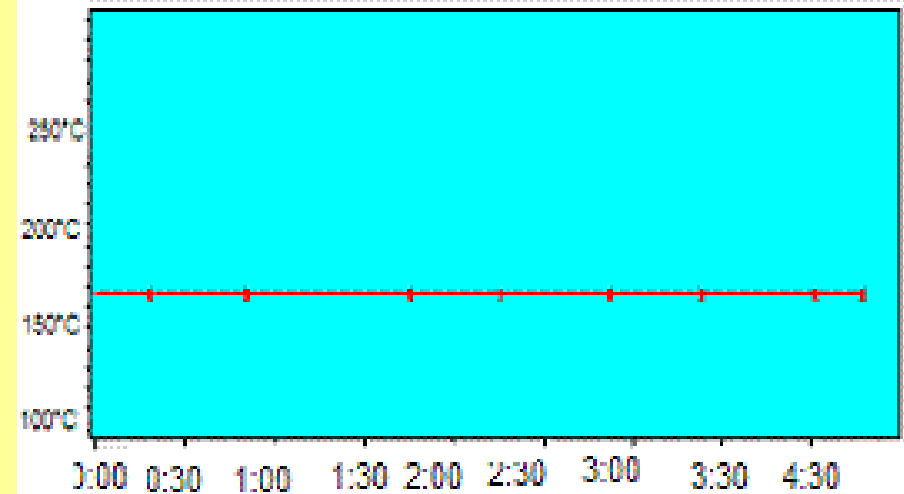
- **Mixing 60% RAP which is at ambient temperature and containing 4% moisture with very hot aggregates is a violent process**
- **Steam develops**
- **Does foaming occur in outer drum of double barrel?**



# Temperature during mixing

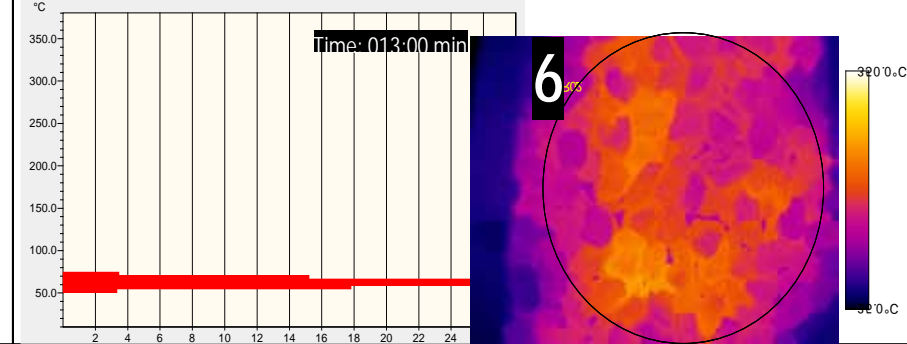
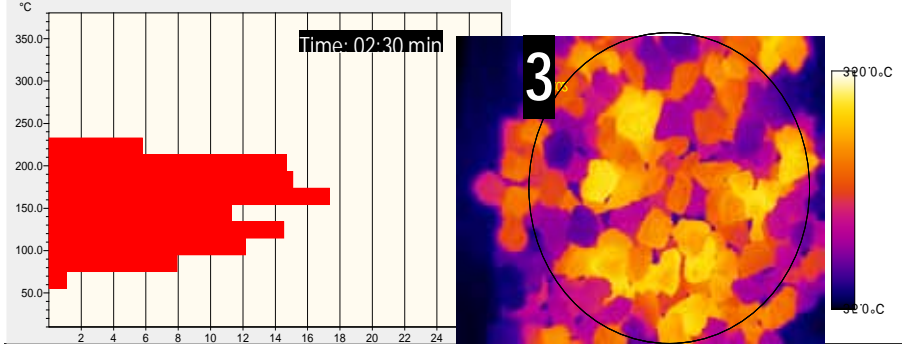
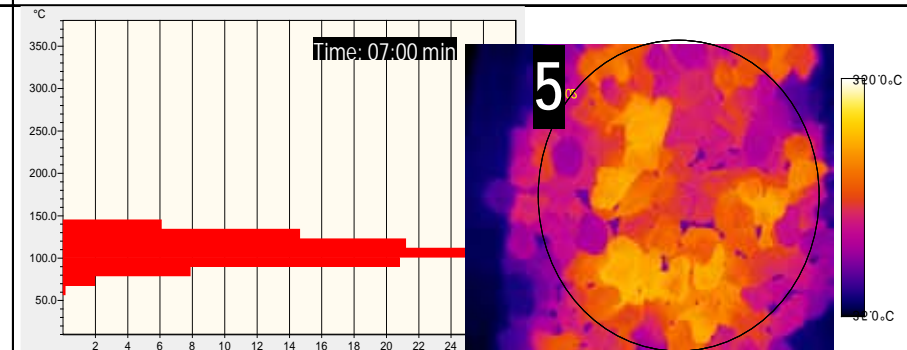
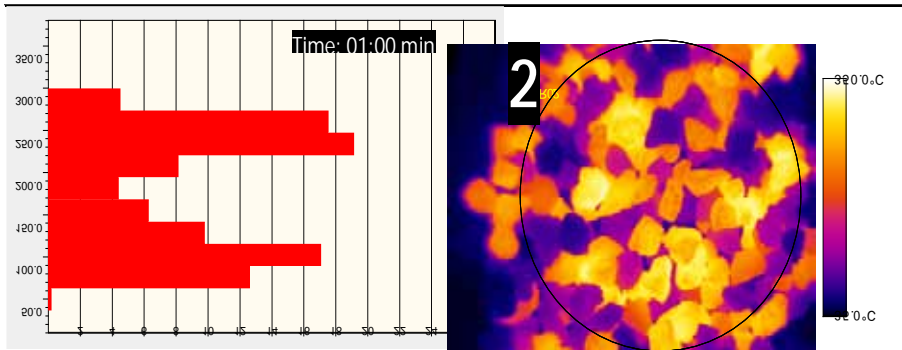
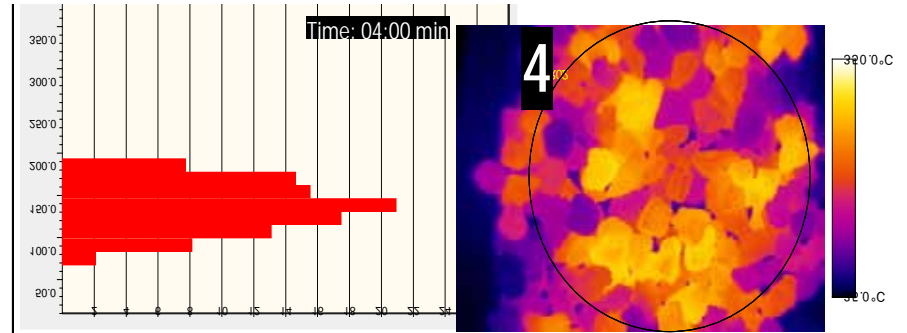
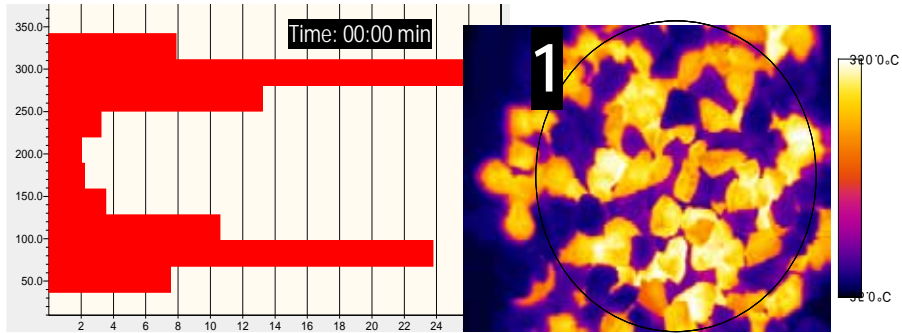


**UPG**



**SM**

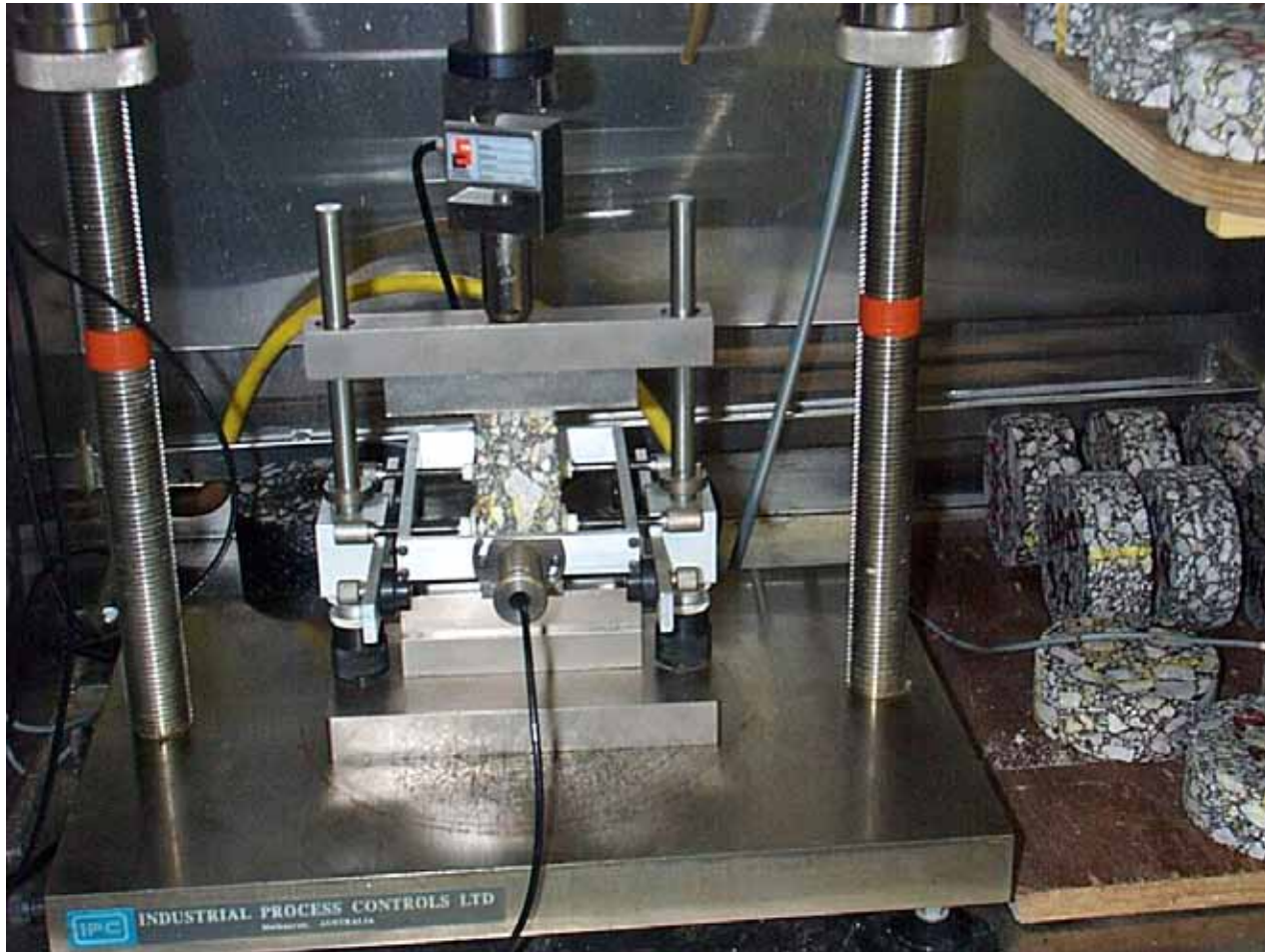
# Temperature Change in Time



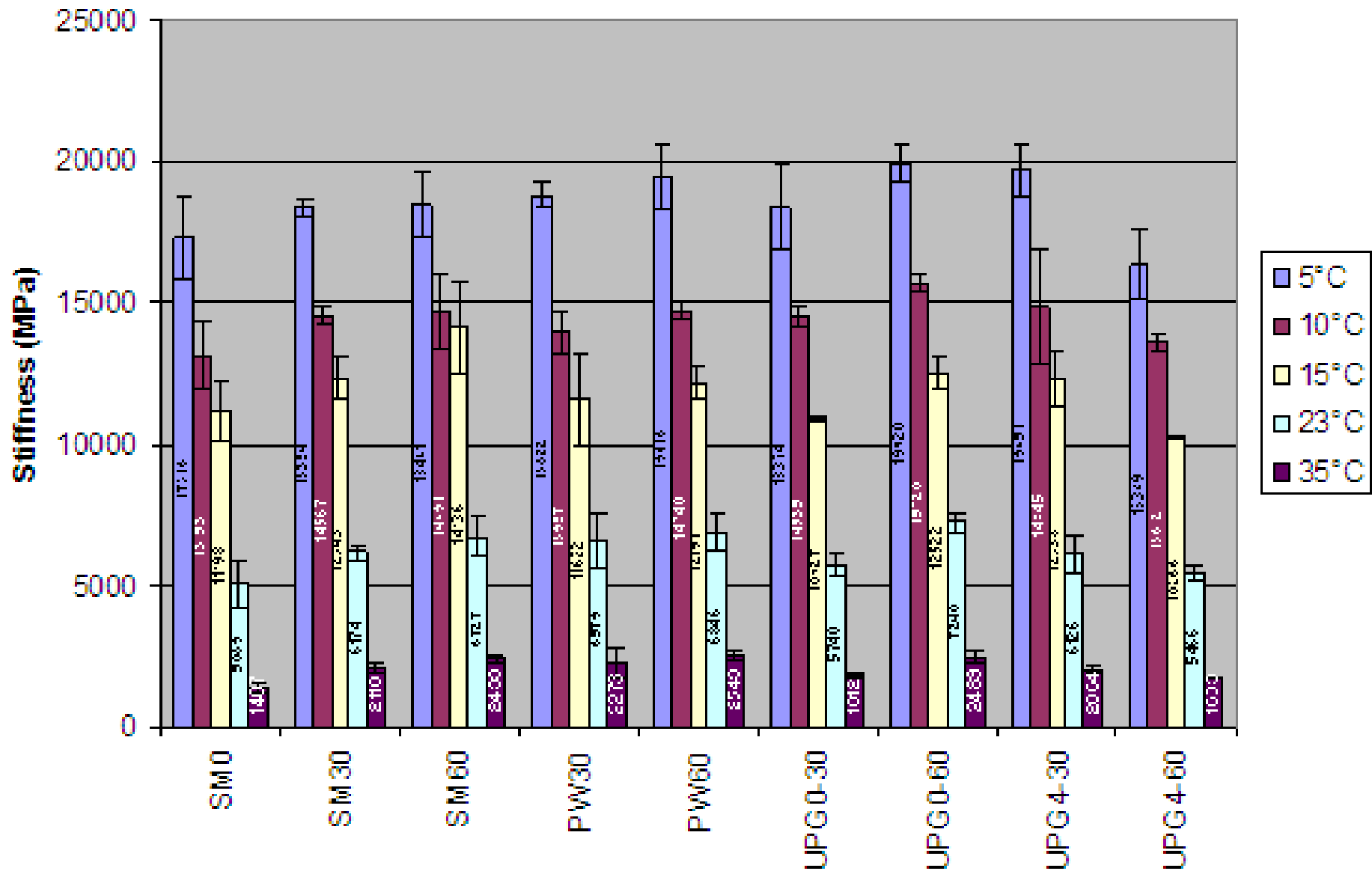


# Stiffness testing

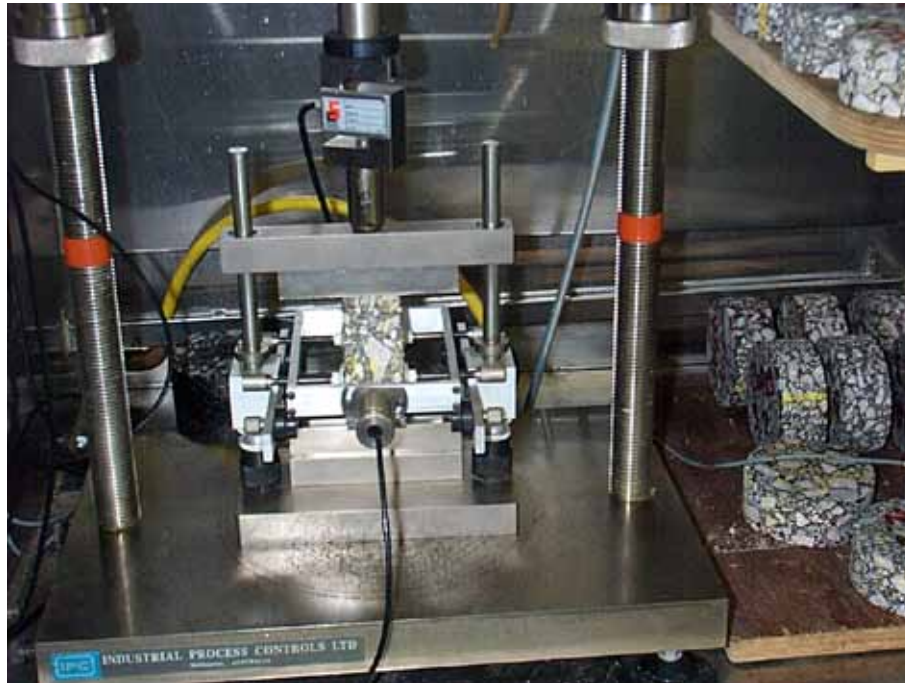
Frequency sweeps at 5, 10, 15, 23 and 35 °C



# Mixture stiffness at 8 Hz



# Fatigue Testing by means of ITT



- 20 °C / 10 Hz
- Only one stress level: 220 kPa
- Reason: limited availability of specimens

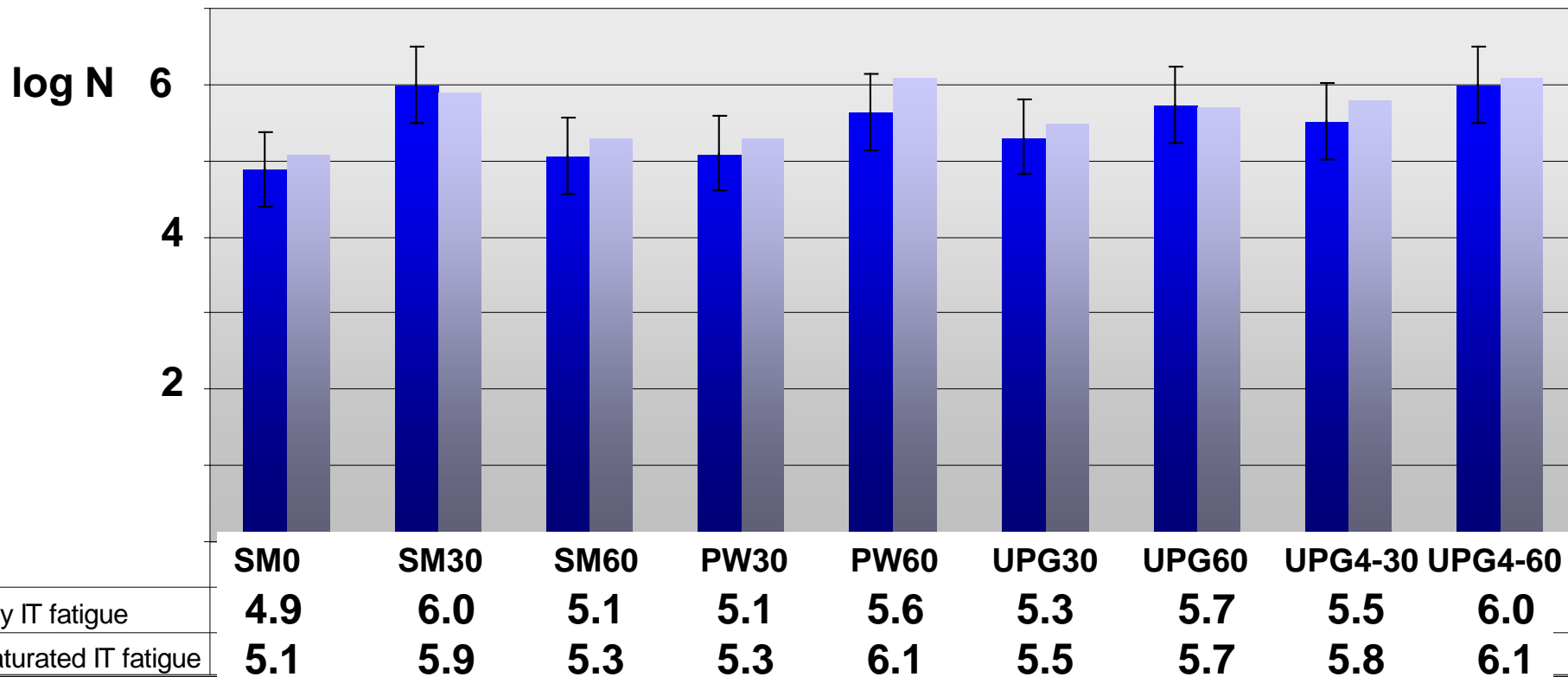
# Fatigue "dry" and "wet"

Fatigue "wet":

fatigue test sample is kept under water during fatigue test

Possible reason:

low void content of mixture (appr. 3%)



# Conclusions

- The amount of RAP as well as its moisture content does not have negative effects on the mechanical properties of the investigated recycled mixtures.
  - Even when the virgin aggregate is preheated to (very) high temperatures there seems to be no negative effect.
  - It takes quite a while for relatively cool RAP to take the same temperature as the entire mixture when mixed with super heated aggregates.
  - Effect of shorter mixing times on the mechanical characteristics of the recycled mixtures should be studied.
  - The ADBM mixing process is very difficult to simulate in the laboratory.
  - The UPG method allows studying the effect of mixing super heated aggregates with cool, moist RAP on the mechanical properties of recycled mixtures.
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**Thank you for your attention**